

1 **Method for Assessment of Modeling Quality for Asphalt Dielectric Constant to Density**
2 **Calibration**

3 **Trevor Steiner; Kyle Hoegh, Ph.D.; Eyoab Zegeye Teshale, Ph.D.; Shongtao Dai, Ph.D.**

4 **ABSTRACT**

5 Traditional measures of asphalt compaction rely primarily on random cores which only
6 measure a small fraction of the pavement. Recently, it has been shown that the use of ground
7 penetrating radar can be used as a non-destructive means for continuous assessment of asphalt
8 compaction. A proposed HD model has been successful in predicting air void content within
9 typically achieved field compaction levels but has reduced accuracy at the extremes. This paper
10 proposes an enhanced, MnDOT model to address this issue. In order to quantify improvement of
11 the MnDOT model, a method for assessment of modeling quality is proposed. The procedure is
12 based on the accuracy of fits when run through a Monte Carlo simulation. The developed
13 procedure shows that the MnDOT model has improved accuracy-- with 0.74% air void variation
14 at a dielectric of 4 compared to 3.83% for the HD fit. Additionally, the MnDOT model is more
15 stable for replicate days of the same mix design and falls within the uncertainty of more of the
16 field cores across several projects than the HD model.

17 **Keywords:** GPR, Asphalt Pavement Compaction, Air Voids, Density, Nondestructive Testing,
18 Nondestructive Evaluation,

19 **Trevor Steiner**
20 Student Worker Para Professional
21 Minnesota Department of Transportation
22 Materials and Road Research
23 Maplewood, MN, 55109
24 Email: trevorsteiner@ucsb.edu
25
26 **Kyle Hoegh**
27 Research Scientist
28 Minnesota Department of Transportation
29 Materials and Road Research
30 Maplewood, MN, 55109
31 Email: kyle.hoegh@state.mn.us
32
33 **Eyoab Zegeye Teshale**
34 Research Scientist
35 Minnesota Department of Transportation
36 Materials and Road Research
37 Maplewood, MN, 55109
38 Email: eyoab.zegeye.teshale@state.mn.us
39
40 **Shongtao Dai**
41 Research Operations Engineer
42 Minnesota Department of Transportation
43 Materials and Road Research
44 Maplewood, MN, 55109
45 Email: Shongtao.dai@state.mn.us

1 INTRODUCTION

2 Recent developments in methods for asphalt compaction evaluation suggest that ground
3 penetrating radar (GPR) can be effectively employed to nondestructively test the relative
4 compaction of placed pavement. This technology has advanced from non-contact horn antennas
5 (Saarenketo & Roimela, 1998) or other methods such as step-frequency array-based systems
6 (Hoegh, Khazanovich, Dai, & Yu, 2015; Leng & Al-Qadi, 2014; Scott, Gagarin, Mills, &
7 Oskard, 2006; Shangguan & Al-Qadi, 2015) to smaller, dipole-type antennas that can accurately
8 measure the dielectric constant of a placed asphalt mixture (Wilson & Sebesta, 2015). These
9 antennas can be placed on a push-cart or a vehicle mount to allow for continuous assessment of
10 the placed pavement's air void content. The procedure for calculating dielectric constant values
11 using GPR antennas uses the surface reflection method. This method is based on measuring the
12 reflection amplitude of the air/asphalt interface. The amplitude of the reflection from air to the
13 asphalt surface, as compared to the incident amplitude (represented by the reflection from a
14 metal plate) is then used to determine the bulk dielectric constant of the asphalt. The ability to
15 continuously measure the density of in-place pavement using GPR technologies would make it
16 easier to provide on-site feedback of the paving operations and techniques, as well as a full
17 picture of the pavement compaction quality. Furthermore, giving that that insufficient asphalt
18 density is the most frequent construction-related performance problem (Killingsworth, 2004), the
19 full coverage testing approach made possible by GPR technology would improve the ability to
20 quickly identify density deficient areas and thus determine the service life of the pavement. It is
21 worthwhile noting that, at the present, most quality acceptance programs rely on random coring
22 that measures less than 1% of the produced total asphalt mixture; it is possible for the cores to
23 misrepresent the true compaction of the full pavement.

1 Compaction level and air void content of a compacted pavement are two interchangeable
2 characteristics: the higher the compaction, the lower air void content. Because of this
3 relationship, the air void content level of pavements is typically reported by the relative
4 compaction or relative density parameter (Equation 1) which quantifies the fraction of the
5 pavement that is not air voids (in reference to the theoretical density):

$$6 \qquad \qquad \qquad RD = 1 - AV = \frac{G_{mb}}{G_{mm}} \qquad (1)$$

7 where RD is the relative density of the pavement, AV is the air void content of the
8 mixture, G_{mb} is the bulk specific gravity of the compacted mixture, and G_{mm} is the theoretical
9 maximum specific gravity of the loose mixture. All three components of asphalt mixture (asphalt
10 binder, aggregate and air void) contribute to the measured dielectric constant, and hence several
11 mix characteristic dependent models have been developed to predict the dielectric constant for a
12 specific combination of components (Al-Qadi, Leng, Lahouar, & Baek, 2010). However, these
13 types of models rely in estimating dielectric constants of the aggregate and binder by back-
14 calculation or from the literature (Al-Qadi et al., 2010). As a result, these models are relatively
15 complex to use on a routine basis. Furthermore, the measured mixture dielectric constant is
16 reliant on the various components used for the specific asphalt mixture, for example, aggregate
17 type and distribution in the mixture have great effect on dielectric constant of the mixture
18 (Zegeye Teshale, Hoegh, Shongtao, Giessel, & Turgeon, 2019), thus conversions between
19 dielectric constant and air void content must be determined for each specific mix design .
20 Recently, empirical models, that are better suited to capture the daily variability of asphalt
21 mixture productions than the existing theoretical models (Al-Qadi I. L. and Riad & M., 1996;
22 Böttcher, van Belle, Bordewijk, Rip, & Yue, 1974; Sihvola, 2008) , have been proposed to

1 facilitate the creation of calibration curves. The most commonly used empirical fits are basic
2 linear and exponential fits (Hoegh et al., 2015; Popik, Lee, Aho, Maser, & Holzschuher, 2010;
3 Saarenketo & Roimela, 1998). The empirical models are calibrated to a specific mix design and
4 require recalibration if significant change in the mix design occurs. Historically, these empirical
5 models have required the collection of field cores from the placed pavement. However, in 2018
6 GSSI and MnDOT researchers showed that asphalt specimens compacted in a Superpave
7 Gyrotory Compactor (SGC) can be efficiently used to create a calibration curve that can convert
8 the measured dielectric constant to the air void content of the pavement (Hoegh, Roberts, Dai, &
9 Zegeye Teshale, 2019; Hoegh, Steiner, Zegeye Teshale, & Dai, 2020). SGC are commonly used
10 by a certified lab technician to fabricate asphalt specimens at specified number of gyrations
11 (based on the expected traffic level for the road) to monitor air void content and density of
12 production mixes. The coreless calibration method described by Hoegh et al. (2019) uses these
13 standard SGC specimens along with additional higher air void content specimens to develop a
14 dielectric to air void content relationship. This new approach provides advantages as it can better
15 represent the overall pavement quality without requiring field cores that are expensive to take
16 and limited in coverage.

17 The empirical fits can be used to reasonably estimate the air void content of a pavement
18 in the 5-10% range but fail to model the extremes of the data. Recently, a new model, the Hoegh-
19 Dai (HD) model has been proposed to better match the data at the extremes and is the current
20 best empirical fit for the experimental data (Hoegh, Dai, Steiner, & Khazanovich, 2018). The HD
21 model also includes physical bounds on the maximum and minimum values possible with the
22 model, preventing the model from predicting negative air void contents or dielectric constants
23 less than 1. Both the exponential and linear models are unbounded. The HD model addresses

1 these issues by allowing for only positive values of air void percentages and having a dielectric
2 of 1 correspond to 100% air voids. The HD model has the form,

$$3 \quad AV = \exp\left(-B\left(D\left|\frac{1}{e-c}-\frac{1}{1-c}\right| - 1\right)\right) \quad (2)$$

4 where the parameters B , D , and C are fit to minimize the sum of the difference of squares
5 between the model and the experimental data. The variables AV , and e represent the modeled air
6 void content and the dielectric constant, respectively.

7 *Deficiency of the HD Model*

8 Although the HD model effectively matches experimental cores with air void contents
9 between 4% and 12%, the HD model fails to match the trends apparent at the farther extremes of
10 the collected field core data. **Figure 1** shows field validation cores and the HD model for trunk
11 highway 371 and trunk highway 15. From the figure, it is clear that the HD model overestimates
12 the slope of the air void vs. dielectric behavior at the extremes of the data. TH 371 was the most
13 convincing data set based on the higher quantity of field validation cores that were taken. In the
14 figure, both of the fits are made using gyratory specimens and then compared to the field
15 validation cores. The HD model is almost within the uncertainty of the validation cores, but the
16 apparent trend at the extremes is not matched, suggesting that the HD model is deficient in its
17 ability to model high air void content data.

18 **OBJECTIVES**

19 An adjusted, logistic model is proposed in this paper to better match field core behavior
20 and allow for improved quality assurance analysis of pavement compaction. This model,
21 thereafter called “MnDOT Model,” expands the range of the fit to cover essentially all
22 reasonable air void contents from a placed pavement. It is currently being used by Minnesota

1 Department of Transportation to assess the ability of GPR to be used in conjunction with
2 coreless calibration from SGC specimens (Hoegh et al., 2019). With improvements to the HD
3 model, a calibration curve can be developed in laboratory using SGC gyratory pucks without
4 taking field cores which damage the pavement. The method for core-free pavement compaction
5 evaluation will be further improved with the new proposed MnDOT model (Hoegh et al., 2019).

6 In addition to providing an improved empirical fit, this paper details a testing procedure
7 to verify fit accuracy and assess the quality of new empirical models. The basis of the proposed
8 method is simulation of random fluctuations in the measured dielectric constant and air void
9 content of the SGC specimens and field validation cores. All experimental measurements have
10 some expected random errors which can come from, for example, the precision of the equipment
11 being used and testing procedure, so it is important that random fluctuations in the measured
12 values do not drastically change the conversion drastically. Accuracy based on random
13 uncertainty will ensure that specific operators or equipment will not result in completely
14 different assessments of the same pavement. To complete an accuracy assessment, a Monte
15 Carlo simulation is conducted (Sokolowski & Banks, 2010). A Monte Carlo simulation simulates
16 numerous test scenarios and forecasts the expected outcomes. To explore the accuracy of the
17 empirical models, the measured values of the air void content and dielectric constant for the SGC
18 specimens were randomly varied within the uncertainty range of each measurement. Next, the
19 values were fit with the empirical model, and the percentage of the simulations where the fit was
20 within the uncertainty of the field validation cores was recorded. By running the simulation on
21 the empirical models, the accuracy of a newly proposed model can also be assessed.

22 Along with Monte Carlo simulations, sensitivity to fluctuations in the mix and parameter
23 sensitivity assessments were conducted. The mix stability was assessed by analyzing two days of

1 paving on the same highway project with an identical target mix design. This is a good scenario
2 to test the stability of the models since there are QA/QC measures in place to ensure that the
3 actual produced mix does not vary significantly from the target properties (ex. target 4% air
4 voids at 60 gyrations). Ideally, the fit would remain constant within these acceptable levels of
5 mix variation. Lastly, the parameter sensitivity was assessed to determine if there were any
6 unnecessary parameters. Additionally, the parameter sensitivity analysis can be used to provide
7 starting values for the parameters. Overall, the combination of these three tests is used to
8 evaluate the proposed MnDOT model. This process is also suggested as a useful process to
9 determine if future models can make further improvement over the existing empirical models.
10 This will be especially useful as density profiling becomes more widespread since the catalogue
11 of available laboratory versus field data that can be used to evaluate the models is increased.

12 **METHODOLOGY**

13 *Proposed MnDOT Density Model*

14 Assessment of field core data shows a reduction in slope at the extreme high and low air
15 void content regions suggesting that the exponential, linear and HD models used to convert
16 dielectric values to air void contents do not correctly predict the behavior of the asphalt at the
17 extremes. **Figure 1** shows that, especially in the case of TH 371, the HD model over predicts the
18 slope at the high air void content data. The observed behavior shows an inflection point with flat
19 slopes near the extremes, which suggests that the data may be better represented by a logistic
20 function of the form (Gottschalk & Dunn, 2005):

$$21 \quad y = d + \frac{a}{\left(1 + \left(\frac{x}{c}\right)^b\right)^g} \quad (3)$$

1 where the parameters b , c , and g correspond to adjustments in the slope and inflection
2 point of the logistic function while d and a correspond to the lower and upper limits of the
3 logistic function, respectively. The logistic function in equation 5 was selected due to the
4 observed trend of decreasing slope at the boundaries of the collected field core data. The
5 parameters in the formula allow for the inflection point and location of these boundaries to be
6 adjusted to the dataset under study. Air void content must be nonnegative, so the lower bound of
7 the function, d , was set to zero. Air has a dielectric constant of 1.0006 at normal pressure and
8 temperature (not 1.0 as commonly assumed). For dielectric values close to that of air, the
9 function must approach 100% air voids (Hector & Schultz, 1936). Considering these limitations,
10 a second, asymptotic term, $\frac{\delta}{(e-1)}$, was added to the fit. The proposed MnDOT model is of the
11 form,

$$12 \quad AV = \frac{a}{\left(1 + \left(\frac{e}{c}\right)^b\right)^g} + \frac{\delta}{(e-1)} \quad (4)$$

13 where AV is the air void content, e is the measured dielectric constant, and the other
14 parameters are defined above. The second portion of the model will force $AV=100\%$ when
15 $e=1.0006$. The remaining parameters are obtained by means of regression (optimization)
16 conducted with the constraints on e and d .

17 The parameters can be optimized using Excel's Solver Add-in or another optimization
18 technique such as Matlab's `fminsearch` function (Mathworks, 2019). Additionally, field hot mix
19 asphalt pavement air void content should not approach 20% in practice. This restriction is valid
20 for Hot Mix Asphalt (HMA) pavements. Any open graded friction courses or porous pavement
21 with extremely high air void content would require modification to the testing method and

1 model. Therefore, the parameter, a in the function is set to have a value of 0.2, corresponding to
2 the approximate physical limit of the possible air void content measured in an HMA field core.
3 With a constant value of a , the value of δ that forces the fit to be 1.0006 at 100% air void
4 consistently had a value of 0.0008 across over 50 fits. Since this parameter remained a constant
5 value, it was decided to fix δ at 0.0008. The model can therefore be rewritten as,

$$6 \quad AV = \frac{0.20}{\left(1 + \left(\frac{e}{c}\right)^b\right)^g} + \frac{0.0008}{(e - 1)} \quad (5)$$

7 The remaining three parameters are optimized by minimizing the sum of the square
8 differences between gyratory puck data and the modeled data. Excel Solver is run with the multi-
9 start option enabled to find the global solution, independent of the initial guesses for the
10 parameters.

11 ***Model Sensitivity Testing***

12 In order to quantitatively assess the quality of improved models, a stability testing routine
13 is proposed. The procedure begins with a Monte Carlo simulation to assess how the random
14 uncertainty in the air void and dielectric constant of the specimens can influence the model
15 stability. The evaluation is made by comparing the 95th percentile confidence windows across the
16 dataset. Next, the variability caused by marginal changes in the mix design is assessed. For this
17 analysis, two or more days of puck data are fit individually, and the fits are compared by
18 calculating the difference between the fits. Again, 95th percent confidence intervals are compared
19 to assess the depth of influence small mix changes have on the model. The final step in the
20 assessment of the model viability is the sensitivity of the fit to changes in the parameters. For this
21 step, each parameter is incremented individually, and the remaining parameters are then refit to

1 the data. This will help to establish cutoff values as well as determine if parameters can be held
2 at a single value to improve the computation time.

3 *Monte Carlo Simulation*

4 Since each of the measurements on the pucks has an associated random uncertainty, a
5 Monte Carlo simulation is suggested to simulate how a model would respond to the innate
6 uncertainty in these measurements. 1000 different simulation sets were conducted based on the
7 collected laboratory puck air void contents and dielectric values. For each simulation, the
8 measured values of dielectric constant and air void content were used as a starting point. The
9 starting dielectric constants and air voids are included in **Table 1**. Each measured value was
10 allowed to vary by $\pm 1.2\%$ for the air void measurements and ± 0.08 for the dielectric
11 measurement. Each simulation set chose a random value within these ranges of the experimental
12 value for each of the 21 collected laboratory pucks. The 1000 sets were then fit to the three
13 models and plotted to show how random uncertainties in the puck measurements would
14 influence the spread of fits that the model would provide. This simulation gives an idea as to how
15 stable each model is to variation in the individual puck dielectric and air void values. This
16 process is recommended as a method for all future models to assess the stability of the proposed
17 models by comparing the percentage of simulated fits that fall within the uncertainty of the field
18 validation cores. This assessment is valuable as it simulates the randomness that could be
19 expected from any given dielectric or air void measurement. It is important that the proposed
20 model can correctly match the core data (or at least be within reasonable uncertainty of the core
21 data) in order for the model to provide a useful dielectric to air void conversion.

22 The Monte Carlo Simulation is intended to simulate variation in air void and dielectric
23 measurements that may be caused by the use of different testing procedures, devices or

1 operators. The simulations are not intended to suggest a function relating air void change due to
2 a change in mix components.

3 *Mix Sensitivity Assessment*

4 It is important that models of laboratory tested pucks do not vary significantly when
5 asphalt mix has typical production mix fluctuations throughout the day. Thus, the proposed
6 coreless calibration model should remain stable with slight mix variations to avoid the
7 requirement of recalibration every time a small fluctuation occurs. A more stable model will
8 reduce the number of recalibrations required for a paving project. Additionally, understanding
9 the influence of mix design changes is useful in suggesting the extent a mix can be altered before
10 recalibration is necessary to assure the fit matches the field data. For a new proposed model, it is
11 suggested that the model be run on several days of test puck data and the fits be compared for
12 how much they vary day-to-day.

13 *Parameter Sensitivity*

14 In order to understand the influence of each of the parameters on the fit quality, a
15 sensitivity study was conducted. The proposed parameter sensitivity analysis fixes the value of
16 one parameter and allows the other parameters to vary while still meeting the constraints for the
17 function (e.g. the dielectric of 100% air must be 1.001). This sensitivity analysis selects one
18 project and fits the specimen data with fixed values of one parameter to determine the
19 parameter's influence on the quality of the fit. This analysis is only intended to determine the
20 reporting accuracy required for each parameter and if any of the parameters can be removed to
21 simplify the model and reduce computation time.

1 **RESULTS**

2 The MnDOT model is applied on the data collected from a project on trunk highway 371
3 (TH 371) in Hackensack, MN. Field cores and production mix were collected from the project
4 for the purpose of coreless calibration and validation of the model fit quality. The uncertainty of
5 the field core measurements is within the acceptable precision range of dielectric measurements
6 of 0.08 (AASHTO PP 98-19, 2019) and the Minnesota Department of Transportation core
7 tolerance for bulk specific gravity (G_{mb}) of 0.03 (Minnesota Department of Transportation,
8 2018). The uncertainty in the G_{mb} is converted to air void content based on the 2.472 maximum
9 specific gravity (G_{mm}) values for the TH371 production days which corresponds to
10 approximately 1.2% air void content. The air void content for the validation field cores was
11 measured using the saturated surface dried method, while the laboratory samples were measured
12 using the AASHTO T331 method (AASHTO T331, 2017). It has been shown that the surface
13 dry method under-predicts the air void contents due to large, interconnecting voids (Cooley,
14 Prowell, Hainin, & Buchanan, 2003). The saturated surface dry bias for large air void contents
15 was shown to be 0.041 in terms of G_{mb} by Cooley et al. This suggests a reduction in air void
16 contents by approximately 1.6% for the high end of the measured field core data. This value is
17 used to correct the high air void content core during the analysis.

18 **Figure 2** shows results from TH 317 project. The traditional exponential model, HD
19 model, and the proposed MnDOT model are used to fit pucker test results. Also, five field cores at
20 random locations were taken to verify the calibration models. The cores were brought to
21 MnDOT laboratory for density determination. The exponential model is the most commonly
22 used in practice, but the HD model shows an improved fit to the experimental data of numerous
23 projects (Hoegh et al., 2018). The HD model represents the data between approximately 4% and

1 12% air void content well. There is, however, a relatively large deviation from the data outside
2 of this air void range. The proposed MnDOT model provides a better fit with the field core data
3 at large air void contents (>12%). In order for a coreless calibration model to be effective at
4 converting measured dielectric values to air void contents, the model needs to be accurate across
5 all expected field data. The incentive structure used by the Minnesota Department of
6 Transportation penalizes pavement that is at high air void contents, so the HD model, which
7 overestimates the slope of the data at high air voids, would result in an excessive penalty as
8 compared to the actually achieved in place compaction. For widespread implementation of GPR
9 as a means of assessing pavement compaction, it is necessary that the method to convert
10 dielectric to air void is accurate throughout all reasonable air void contents.

11 *Example using the Proposed Model: Monte Carlo Simulation*

12 The proposed model was compared to the Hoegh-Dai model, and the traditional
13 exponential model, to evaluate the sensitivity of the models to variation in the measured air void
14 and dielectric constant of the pucks. All models were fit using the same 1000 simulated air void
15 and dielectric values. **Figure 3** shows a comparison between the models and the stability of the
16 models from the expected variation in dielectric and air void measurements. As evident in the
17 figure, the new model has a significantly smaller spread in the high and low dielectric regions
18 with 0.74% air void variation at a dielectric of 4 compared to 7.59% air void variation for the
19 exponential fit and 3.83% air void variation for the HD model.

20 Another assessment of the quality and stability of the models is the ability of the
21 simulated fits to match field cores taken on the project. The stability is assessed using the
22 percentage of the 1000 simulated fits that fall within the uncertainty of the field cores. **Figure 3**
23 shows the field cores taken on the TH 371 project with their associated uncertainties in

1 measurement. All three models correctly fall within uncertainty of the five higher dielectric cores
2 100 percent of the time. But, none of the models are within the lowest dielectric core. As
3 previously stated, this core likely has an underestimated air void content, so the Corelok
4 corrected core is included in the figure (Cooley et al., 2003). When this value is assessed, the
5 three models diverge in their percentage within uncertainty. The MnDOT model has 100% of the
6 fits within the Corelok corrected core while the exponential model has only 21.2% within the
7 core's uncertainty and the HD model has 29.5%. For this analysis, the percentage of fits falling
8 within the uncertainty of the corrected core (shown with a solid red dot in **Figure 3**) can be
9 determined by connecting the ends of the horizontal and vertical error bars. This creates a
10 rectangle of uncertainty for the measured field core. All fits that fall anywhere within this
11 rectangle are considered to be within the uncertainty of the field core measurement. For the cases
12 of the exponential and HD models, only 21.2% and 29.5% of the 1000 simulated fits are within
13 this rectangle of uncertainty. Since the Corelok test method is recommended for high air void
14 content cores (Cooley et al., 2003). Therefore, air void content determined from Corelok is a
15 better representation of the true air void content of the core. This analysis shows that the
16 proposed improved model has significant improvement over the other two models.

17 **Figure 3** depicts the results of the Monte Carlo simulation by displaying all 1000 of the
18 fits created by the method. While **Figure 3** gives a good qualitative view of the improved
19 stability of the MnDOT model, it is also useful to quantitatively assess the spread of values that
20 random puck variation can cause. The 95th percent confidence interval is reported as twice the
21 standard deviation of the fits at each dielectric value. The assumption that the data has a normal
22 distribution is a common assumption made when evaluating the quality of the placed asphalt
23 (Breakah, Kandil, Williams, & Shane, 2007). Many agencies use the percent within limits

1 method to evaluate the quality of the pavement while also allowing for material variation. This
2 method requires the assumption that the compaction follows a normal distribution. Therefore, the
3 use of twice the standard deviation of the fits for the 95th percent confidence interval is valid.
4 The confidence of each fit across the range of reasonable dielectric values is shown in **Figure 4**.

5 All three models perform to a satisfactory level within the central region of the dielectric
6 range of the pucks. The confidence interval is within the uncertainty of the air void
7 measurements, so the variation in the fits is small enough that it will have little to no influence
8 on the predicted air voids around this region. At the low end of the reasonable dielectric
9 measurements for the project, the new model has a drastically improved confidence level, on the
10 order of a 5 times smaller spread. This increase in stability is important as it indicates that the
11 new model will be more stable to random uncertainty in the laboratory puck data, particularly for
12 pucks or cores with large air void content. This is especially critical in assisting the feasibility of
13 widespread implementation of the density profiling method in a field environment where the
14 number of fabricated asphalt sample pucks used for calibration of the model may be limited.

15 *Example using the Proposed Model: Mix Sensitivity Assessment*

16 The TH 371 project had several days of testing with variation in the mix from day-to-day,
17 so the models were fit to each testing day and assessed for stability across mixes. The objective
18 would be for the fit to match the field core data well without a dependence on the day of paving
19 that the laboratory pucks were made. For the TH 371 project, the field cores were taken on
20 October 1st and 6th. The results from the laboratory pucks that were tested on these days were fit
21 to investigate how well the model fits between two days of paving. **Figure 5** displays the
22 models' sensitivity to slight mix changes and other daily fluctuations that could be expected.

1 Similar to the assessment completed for the puck uncertainty sensitivity, the 95th percent
2 confidence interval is also assessed for changes to the mix design. **Figure 6** depicts the
3 confidence window for the HD and new model. Again, the new model has an improved stability
4 when compared to the HD model. Further assessment can be completed by determining the
5 acceptable change in mix that can still fall within the confidence window to allow the same
6 calibration curve to be used.

7 *Example using the Proposed Model: Parameter Sensitivity*

8 As discussed above, the parameter a was fixed to a value of 0.2, corresponding to the
9 maximum expected air void content of a field core. In order to assess whether this assumption
10 allowed for an optimal fit, a was allowed to vary from a value of 0.15 to 0.5, and the densities
11 obtained from the collected gyratory pucks were used for regression analysis. The various fits
12 were compared to the collected field core density data. **Figure 7a** shows the resulting regression
13 curves of various a values. It can be seen that the specific a value of 0.2 sets the upper asymptote
14 of the logistic function. Too low of a value creates excessive curvature that does not fit the trend
15 observed in the cores at high end of air void content, and too high of a value increases the slope
16 of the fit at the low dielectric region, fitting the data poorly.

17 A similar assessment was completed for the remaining parameters. **Figures 7b-d** show
18 all of the remaining parameters and the fits resulting from fixing each parameter at a specific
19 value while a is set to 0.2. One of the most important trends evident from this analysis is that the
20 parameter g can be varied significantly, and the other parameters adjust to result in an
21 insignificant amount of variation in the fit, as shown in **Figure 7d** by the small change in the fit
22 caused by a three orders of magnitude change in g . This suggests that it could be possible to fit
23 the data without changing the value of g . Fixing the value of g results in a slight increase in the

1 sum of the difference of squares for the data set, so the optimal solution is still found by allowing
2 g to vary.

3 Unlike g , the parameter c plays a significant role in the quality of the fit. The parameter c
4 increases the slope of the fit and can result in overestimation of the air void content of the
5 pavement for low dielectric values.

6 The range of values presented in this analysis should offer reasonable starting points (or
7 initial guesses) that are required to complete the parameter optimization. It is important to note,
8 however, that the parameters can vary between mix designs, so there may be a specific mix
9 design that results in parameters exceeding the expected range.

10 *Model Verification on Multiple Projects*

11 In order to confirm that the proposed model is not uniquely suited for the chosen
12 highway, TH 371, the ability of the new improved MnDOT model was used to match field cores
13 collected on TH 60, TH 55 and TH 61. The four selected projects have significantly different
14 mix designs and aggregate sources, so the model is tested to see if it can remain flexible enough
15 to handle various asphalt mixes. For brevity, only the two majority aggregate sources are listed
16 for each project. The remaining aggregate sources are available upon request. TH 371 has the
17 mix designation SPWEA340C with $\frac{1}{2}$ inch maximum aggregate size and PG 58-34 binder. The
18 majority aggregate source components were 30% Powers BA Sand and 22% Powers $\frac{1}{2}$ Rock. TH
19 60 uses mix designation SPWEB440 with $\frac{3}{4}$ inch maximum aggregate size and PG 58H-28
20 (MSCR) binder. The majority source components were 33% SRP WMS (36) and 21% SRP $\frac{3}{4}$
21 DF (18). TH 55 has mix designation SPWEA340 with $\frac{1}{2}$ inch maximum aggregate size and PG
22 58-34 binder. The majority source components were 29% Naak Nat Fine 3A-BA19-0029 and
23 22% Naak Washed Cr. Fines 3A-BA19-0028. TH 61 uses mix SPWEB440 with $\frac{3}{4}$ inch

1 maximum aggregate size and PG 58S-28 (MSCR) binder. The majority source components were
2 38% Doane ¾ Bit Rock and 34% Doane Man Sand.

3 Although the mix components and specific mix designs are included in this manuscript, it
4 is important to note that the calibration methodology accounts for the aggregate dielectric
5 properties as part of the bulk production mix from the specific day of paving where DPS data
6 was collected. Based on the results shown in **Figure 8**, the regression curves on the three projects
7 correspond very well with the field core density results. The predictions are within the
8 uncertainty of the collected field cores, suggesting that the model improves on the current state
9 of the art models and is applicable to more than just the TH 371 project.

10 **CONCLUSION**

11 A new model is proposed to establish calibration relationship between dielectric
12 measurement and field HMA compaction density. The MnDOT model improves on the
13 exponential and HD models in its ability to correctly convert collected GPR field data to in place
14 air voids, especially at the extremes. The incentive structure used by the Minnesota Department
15 of Transportation (and many other state departments of transportation) penalizes pavement that is
16 at high air void contents, so the HD model, which overestimates the slope of the data at high air
17 voids, could result in an excessive penalty as compared to the actually achieved in place
18 compaction. The increased accuracy of the MnDOT model will reduce misrepresentation of the
19 actual compaction of the pavement. The improvements of the new model were evaluated and
20 verified using a novel, statistical analysis procedure. The statistical approach employed Monte
21 Carlo simulations to assess the variability in the fit that can occur due to slight fluctuations in the
22 measured puck values. This assessment shows that the MnDOT model falls within expected
23 uncertainty of all of the field cores (after correcting the highest air void content core for the

1 saturated surface dry method) while the HD model only falls within 29.5% of the time.
2 Additionally, assessment of the spread of the Monte Carlo simulated fits shows that the MnDOT
3 fit has the least variation across all expected field air void content ranges. Next, the models are
4 tested across the same asphalt mix design on different production days to determine if slight
5 variation in the mix would make the conversion no longer useful. This analysis also shows
6 improved stability for the MnDOT model compared to the HD model. Field core validation
7 results showed that the stability of the MnDOT model allowed for accurate prediction of in-place
8 air voids even when the mix from a different day of production was used to convert the GPR
9 collected data to air voids. The final step in the model testing procedure is to determine the
10 sensitivity of the models to their parameters. This step verifies that all parameters are necessary
11 and contribute to the quality of the fit that is created. This complete testing procedure confirms
12 that the MnDOT model is a better tool for use in coreless calibration of collected GPR data to air
13 void contents than the currently available models. The techniques used to complete this
14 assessment can also be employed to assess future proposed models or in assessing the magnitude
15 of mix design change that is required for recalibration of the conversion using SGC specimens.

16 Future research efforts will focus on obtaining more of the high and low air void content
17 field cores to further verify the improvements made by the MnDOT model. The assessment
18 presented in this report was supported by only three field cores at the extremes of the measured
19 air void content. Thus, future work will attempt to sample more cores with greater than 12% air
20 void content and less than 4% air void content. For the high air void content cores, field cores
21 can be taken from low compaction regions such as pavement on road shoulders. On the other
22 extreme, the real-time display of the DPS can be used to identify regions with very high
23 compaction for coring.

1 **DATA AVAILABILITY**

2 All data, models, or code generated or used during the study are available from the
3 corresponding author by request. This includes Microsoft Excel Macros to run fit optimization and
4 Matlab code to create the Monte Carlo simulations and visualize the results.

5 **ACKNOWLEDGMENTS**

6 The authors would like to acknowledge Steve Cooper, Tom Yu, and the Federal Highway
7 Administration as well as Glenn Engstrom, Jeff Brunner and the NRRA for partial funding of the
8 work presented in this paper. Ray Betts, Mercedes Maupin, Joseph Voels, and Thomas Boser
9 from MnDOT were all instrumental in the gyratory puck fabrication. Karl Olson from MNDOT
10 programmed some of the analysis software that allowed for the results presented in this paper.
11 Roger Roberts and GSSI provided the equipment used for the presented study and developed the
12 coreless calibration method used for the bulk of analysis in this study. Curt Turgeon has been
13 instrumental in driving the research toward solutions that allow the technology to be deployed in
14 paving projects. The authors have worked with Curt consistently in identifying barriers to
15 implementation and brainstorming paths to addressing critical issues.

16 **AUTHOR CONTRIBUTIONS**

17 The authors confirm contribution to the paper as follows: study conception and design:
18 Trevor Steiner, Kyle Hoegh, Eyoab Zegeye Teshale, and Shongtao Dai, data collection: Trevor
19 Steiner, Kyle Hoegh, Eyoab Zegeye Teshale, analysis and interpretation of results: Trevor
20 Steiner, Kyle Hoegh, Eyoab Zegeye Teshale, and Shongtao Dai; draft manuscript preparation:
21 Trevor Steiner and Kyle Hoegh. All authors reviewed the results and approved the final version
22 of the manuscript.

23

1 **REFERENCES**

2 AASHTO PP 98-19. (2019). Asphalt Surface Dielectric Profiling System Using Ground
3 Penetrating Radar. American Association of State and Highway Transportation Officials.
4 Technical Subcommittee Number 5c. New Provisional Standard.

5 AASHTO T331. (2017). Bulk Specific Gravity and Density of Compacted Hot Mix Asphalt Using
6 Automatic Vacuum Sealing Method. Washington D.C.: American Association of State
7 Highway and Transportation Officials.

8 Al-Qadi I. L. and Riad, & M., S. (1996). *Characterization of Portland Cement Concrete:*
9 *Electromagnetic and Ultrasonic Measurement Techniques. Report Submitted to the National*
10 *Science Foundation.*

11 Al-Qadi, I. L., Leng, Z., Lahouar, S., & Baek, J. (2010). In-place hot-mix asphalt density
12 estimation using ground-penetrating radar. *Transportation Research Record, 2152(2152),*
13 19–27. <https://doi.org/10.3141/2152-03>

14 Böttcher, C. J. F., van Belle, O. C., Bordewijk, P., Rip, A., & Yue, D. D. (1974). Theory of Electric
15 Polarization. *Journal of The Electrochemical Society, 121(6), 211C.*
16 <https://doi.org/10.1149/1.2402382>

17 Breakah, T. M., Kandil, A., Williams, R. C., & Shane, J. S. (2007). Implementing Percent within
18 Limits for Hot Mix Asphalt. In *Proceedings of the 2007 Mid-Continent Transportation*
19 *Research Symposium.*

20 Cooley, L. A., Prowell, B. D., Hainin, M. R., & Buchanan, M. S. (2003). BULK SPECIFIC
21 GRAVITY ROUND-ROBIN USING THE CORELOK VACUUM SEALING DEVICE.
22 *NCAT Report 02-11, (December).*

- 1 Gottschalk, P. G., & Dunn, J. R. (2005). The five-parameter logistic: A characterization and
2 comparison with the four-parameter logistic. *Analytical Biochemistry*, 343(1), 54–65.
3 <https://doi.org/10.1016/j.ab.2005.04.035>
- 4 Hector, L. G., & Schultz, H. L. (1936). The dielectric constant of air at radiofrequencies. *Journal*
5 *of Applied Physics*, 7(4), 133–136. <https://doi.org/10.1063/1.1745374>
- 6 Hoegh, K., Dai, S., Steiner, T., & Khazanovich, L. (2018). Enhanced model for continuous
7 dielectric-based asphalt compaction evaluation. *Transportation Research Record*.
8 <https://doi.org/10.1177/0361198118794068>
- 9 Hoegh, K., Khazanovich, L., Dai, S., & Yu, T. (2015). Evaluating asphalt concrete air void
10 variation via GPR antenna array data. *Case Studies in Nondestructive Testing and Evaluation*,
11 3, 27–33. <https://doi.org/10.1016/j.csndt.2015.03.002>
- 12 Hoegh, K., Roberts, R., Dai, S., & Zegeye Teshale, E. (2019). Toward Core-Free Pavement
13 Compaction Evaluation: An Innovative Method Relating Asphalt Permittivity to Density.
14 *Geosciences*, 9(7), 280. <https://doi.org/10.3390/geosciences9070280>
- 15 Hoegh, K., Steiner, T., Zegeye Teshale, E., & Dai, S. (2020). *Coreless Compaction Assessment-*
16 *MnDOT 2019 Case Studies. Transportation Research Record*.
- 17 Killingsworth, B. M. (2004). Research Results Digest 291: Quality Characteristics for Use with
18 Performance-Related Specifications for Hot Mix Asphalt. In *Transportation Research Board*,
19 *National Research Council*.
- 20 Leng, Z., & Al-Qadi, I. L. (2014). An innovative method for measuring pavement dielectric
21 constant using the extended CMP method with two air-coupled GPR systems. *NDT and E*

1 *International*, 66, 90–98. <https://doi.org/10.1016/j.ndteint.2014.05.002>

2 Mathworks. (2019). Find Minimum of Unconstrained Multivariable Function Using Derivative-
3 Free Method.

4 Minnesota Department of Transportation. (2018). Minnesota 2018 Standard Specifications,
5 Specification Section 2360.2.

6 Popik, M., Lee, H., Aho, B., Maser, K., & Holzschuher, C. (2010). Using ground penetrating radar
7 for evaluation of asphalt density measurements. In *Transportation Research Board*.

8 Saarenketo, T., & Roimela, P. (1998). Ground Penetrating Radar Technique in Asphalt Pavement
9 Density Quality Control. In *7th International Conference on Ground Penetrating Radar*.

10 Scott, M. L., Gagarin, N., Mills, M. K., & Oskard, M. (2006). Step frequency ground penetrating
11 radar applications to highway infrastructure measurement and system integration feasibility
12 with complementary sensors. In *AIP Conference Proceedings*.
13 <https://doi.org/10.1063/1.2184716>

14 Shangguan, P., & Al-Qadi, I. L. (2015). Calibration of FDTD simulation of GPR signal for asphalt
15 pavement compaction monitoring. *IEEE Transactions on Geoscience and Remote Sensing*.
16 <https://doi.org/10.1109/TGRS.2014.2344858>

17 Sihvola, A. (2008). Electromagnetic Mixing Formulas and Applications. *IEEE Circuits and*
18 *Devices Magazine*. <https://doi.org/10.1109/MCD.2002.981301>

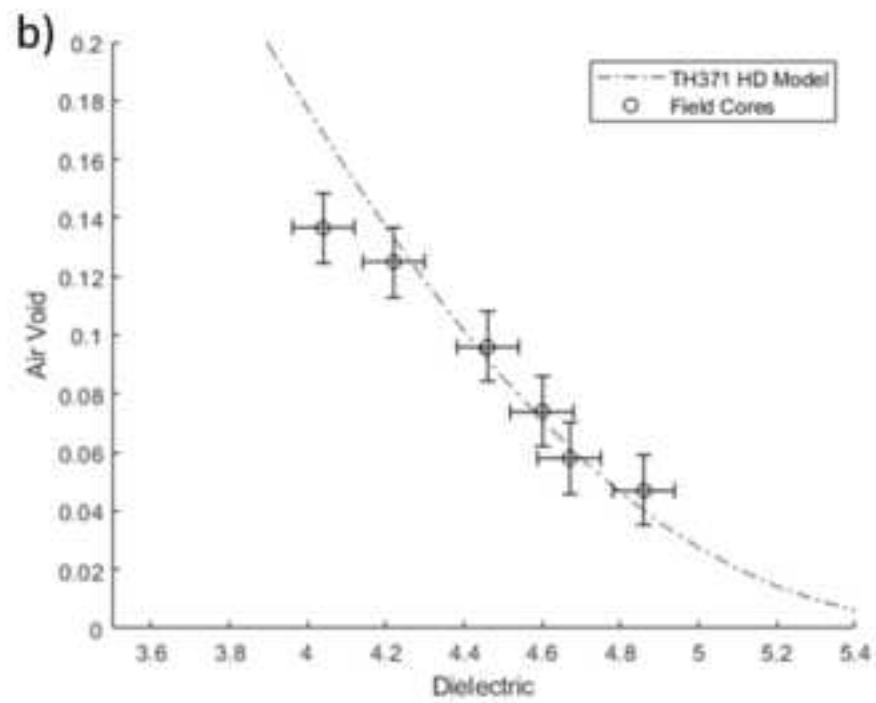
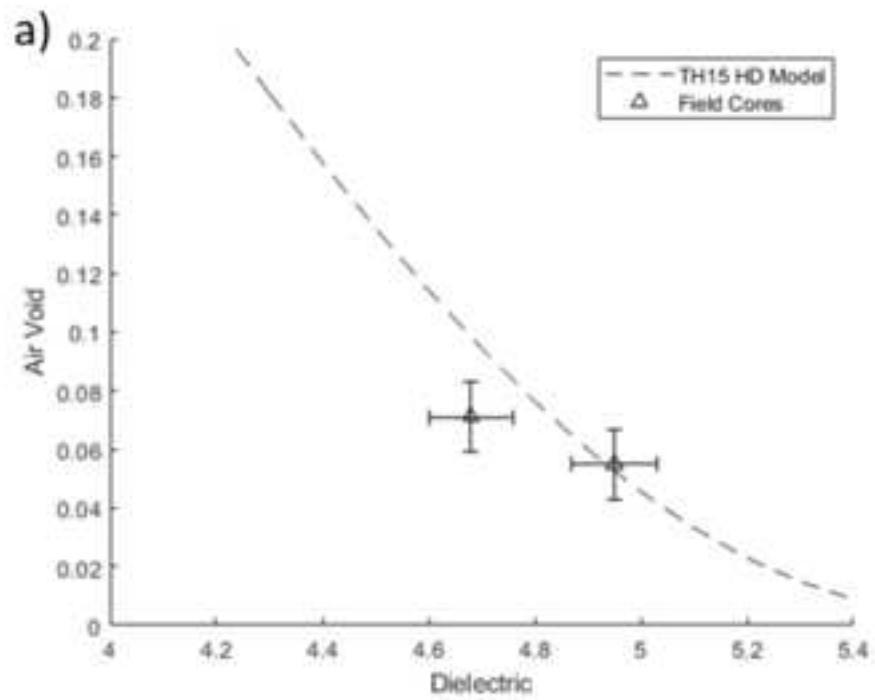
19 Sokolowski, J. A., & Banks, C. M. (2010). *Modeling and Simulation Fundamentals: Theoretical*
20 *Underpinnings and Practical Domains. Modeling and Simulation Fundamentals: Theoretical*
21 *Underpinnings and Practical Domains*. <https://doi.org/10.1002/9780470590621>

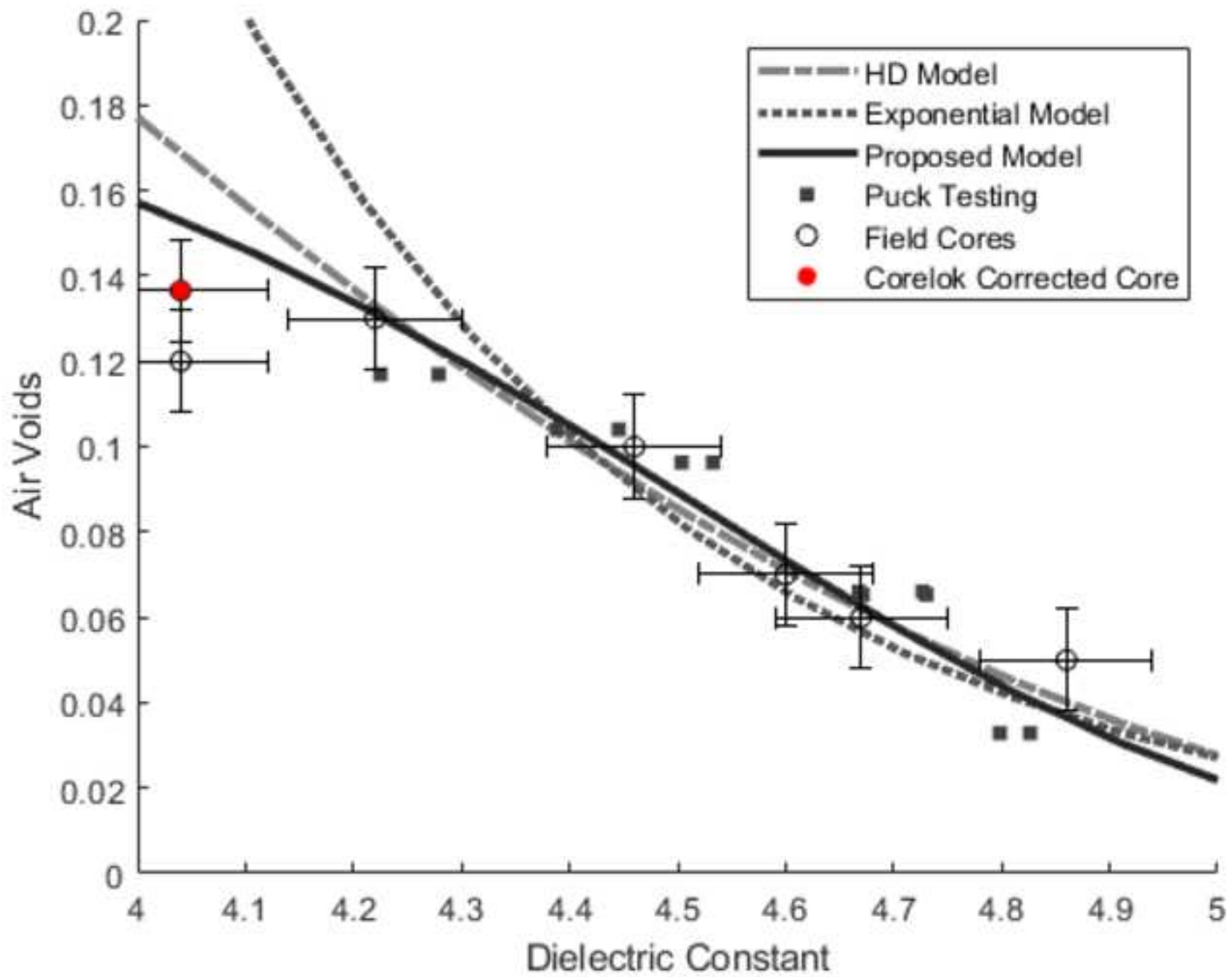
1 Wilson, B. T., & Sebesta, S. (2015). Comparison of Density Tests for Thin Hot-Mix Asphalt
2 Overlays. *Transportation Research Record: Journal of the Transportation Research Board*.
3 <https://doi.org/10.3141/2504-17>

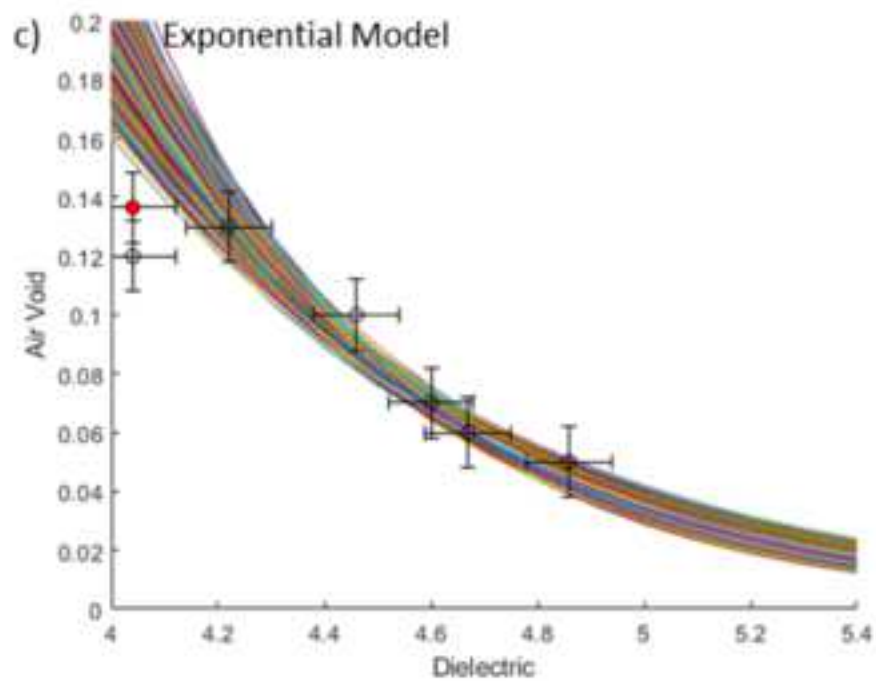
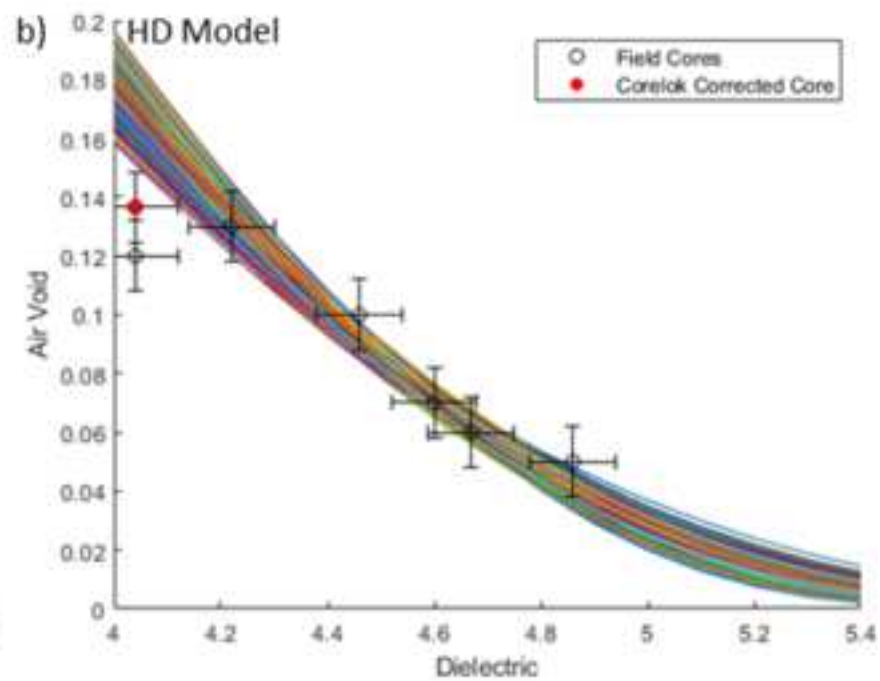
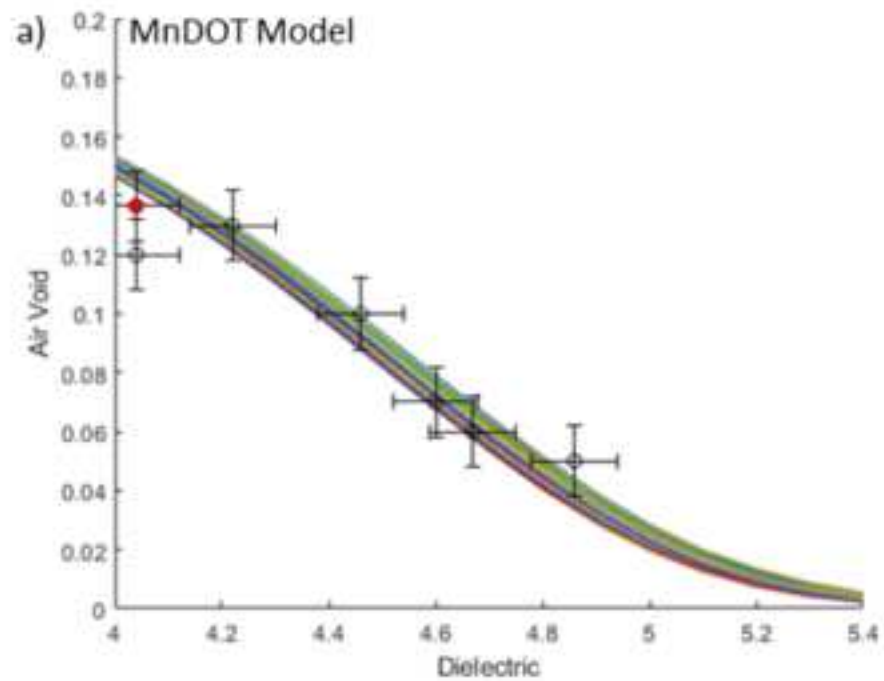
4

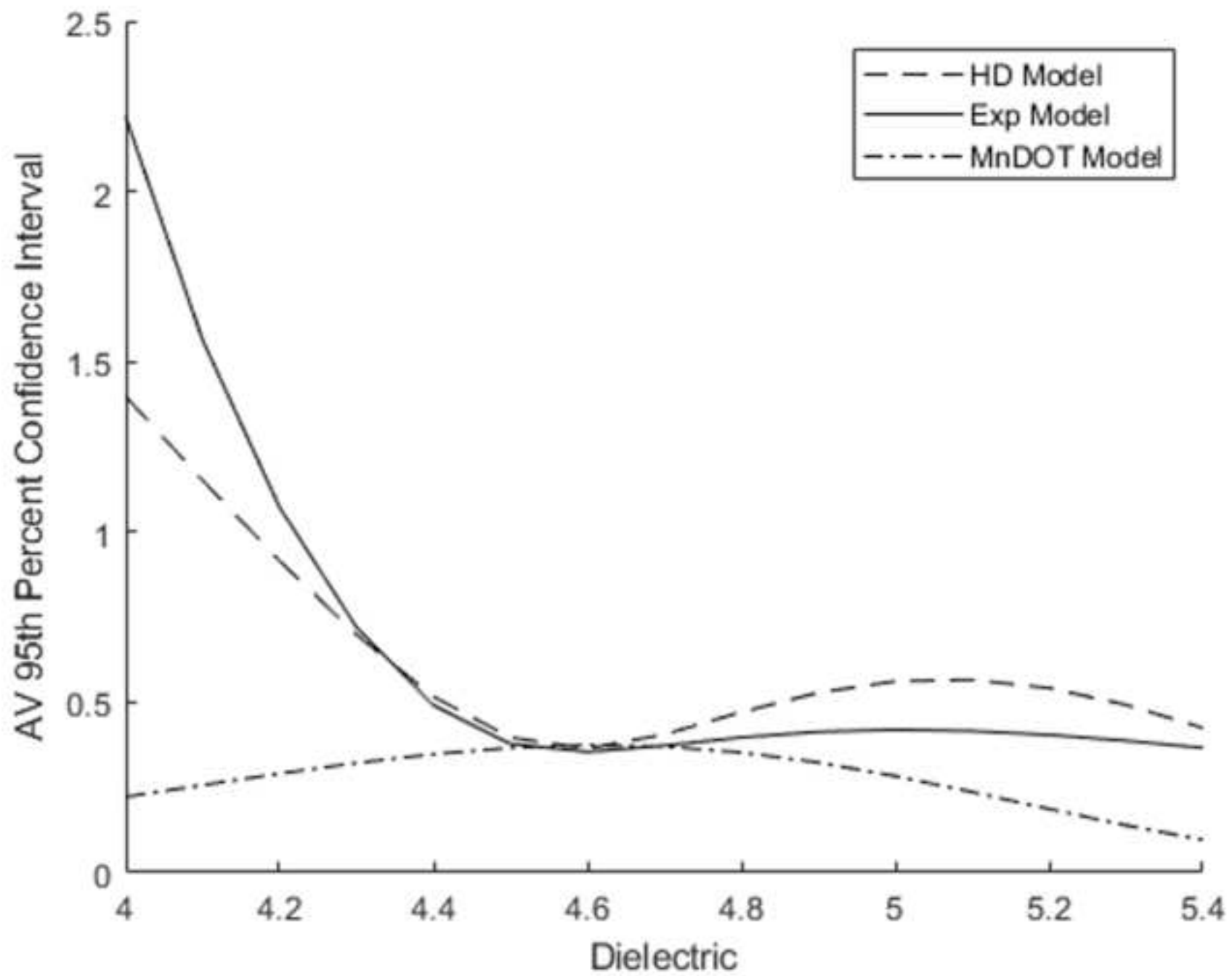
Table 1: Starting Values for the Monte Carlo Simulation

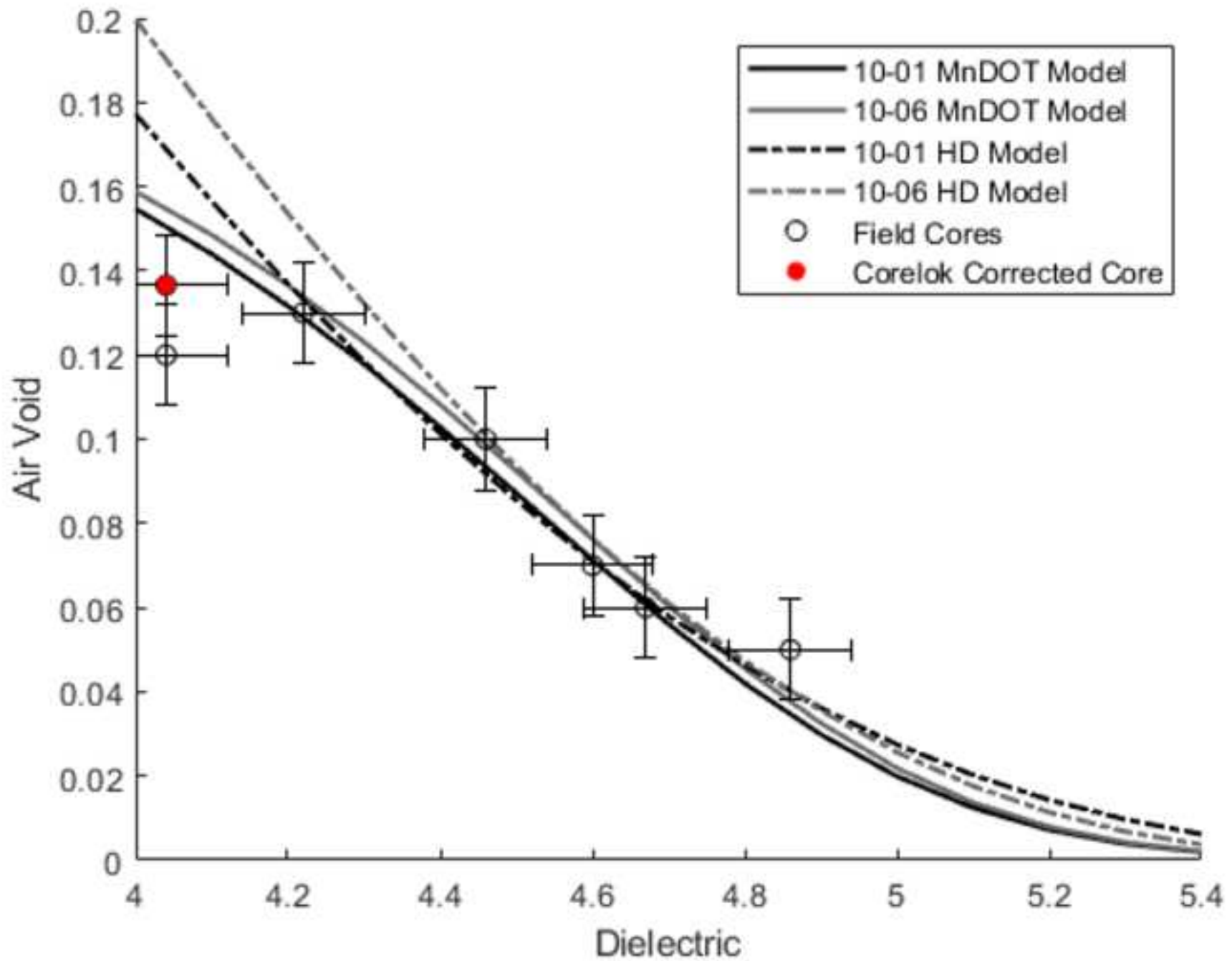
Air void	Dielectric constant
3.29%	4.80
3.29%	4.80
3.29%	4.80
3.29%	4.83
3.29%	4.83
3.29%	4.83
6.53%	4.67
6.53%	4.67
6.53%	4.73
6.60%	4.67
6.60%	4.67
6.60%	4.73
9.63%	4.53
9.63%	4.53
9.63%	4.50
11.69%	4.28
11.69%	4.23
11.69%	4.23
10.40%	4.39
10.40%	4.45
10.40%	4.45

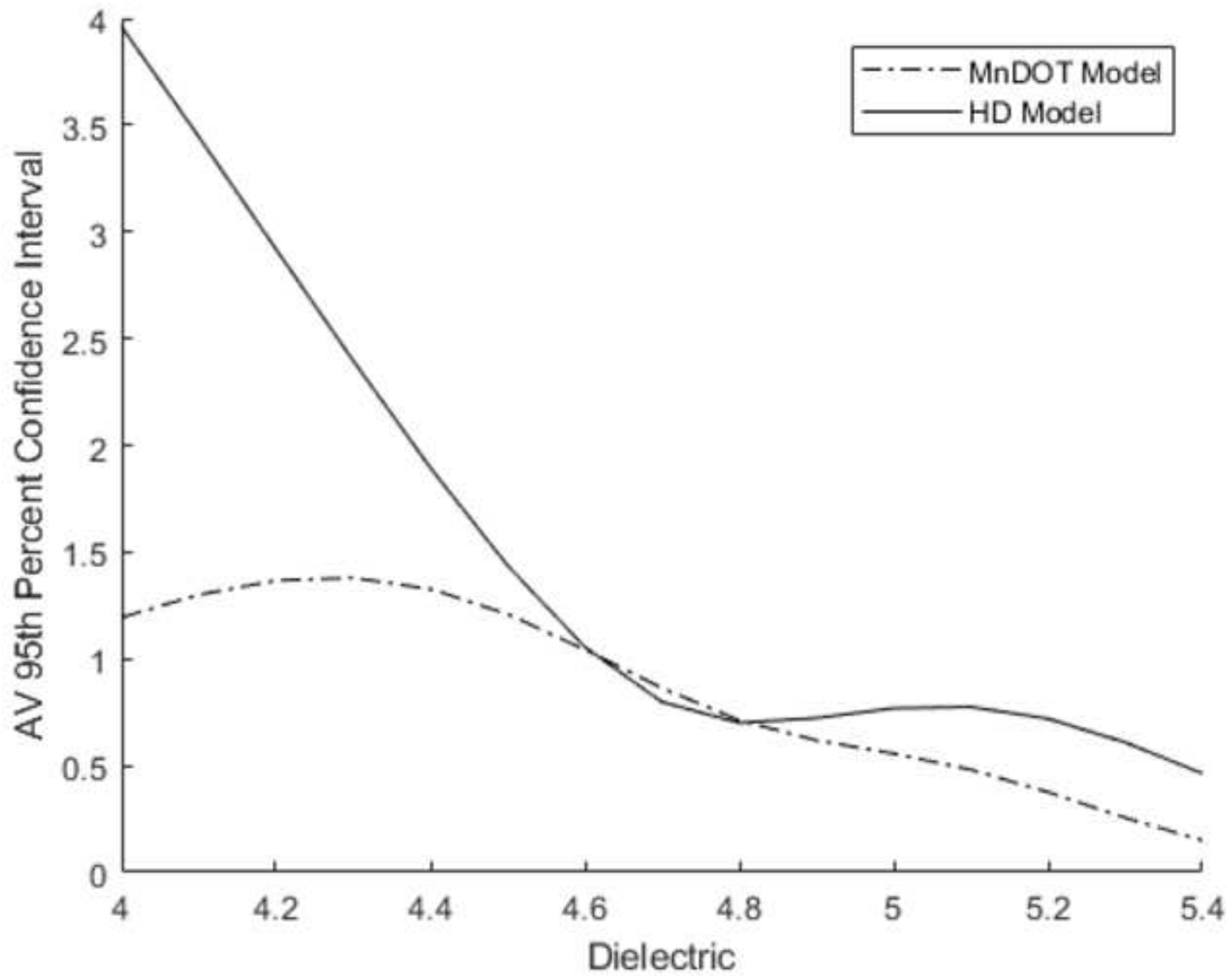


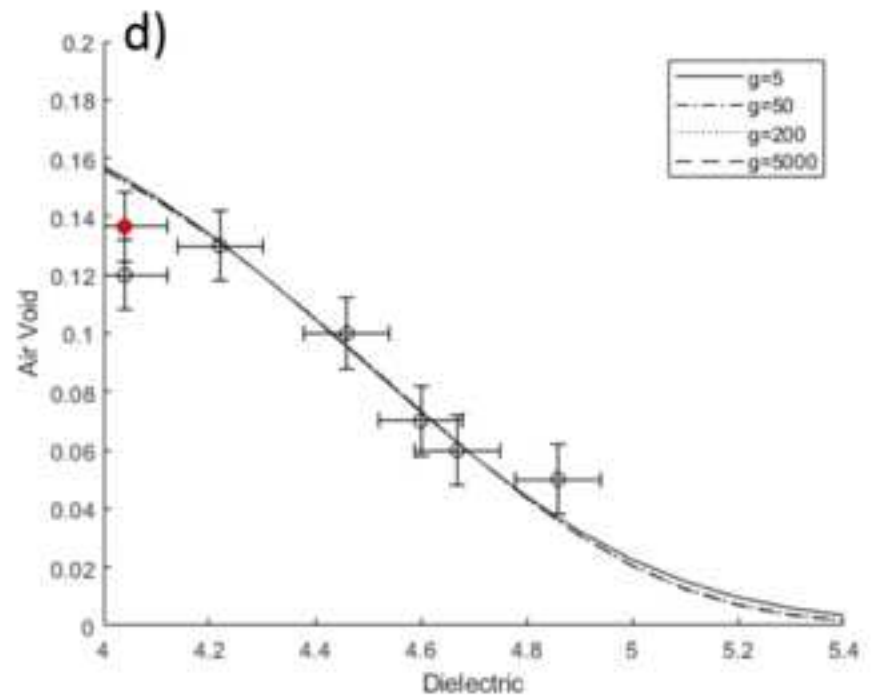
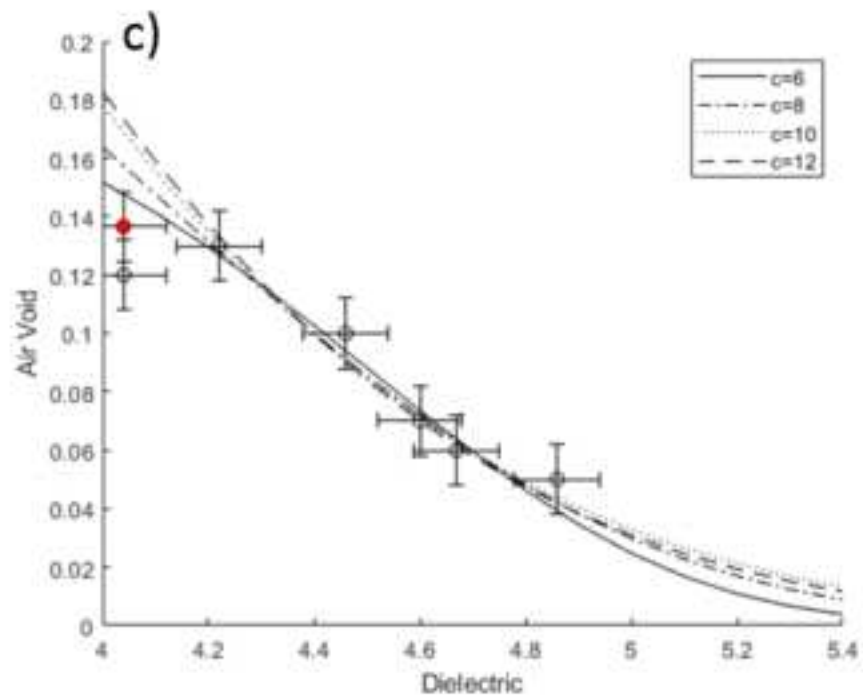
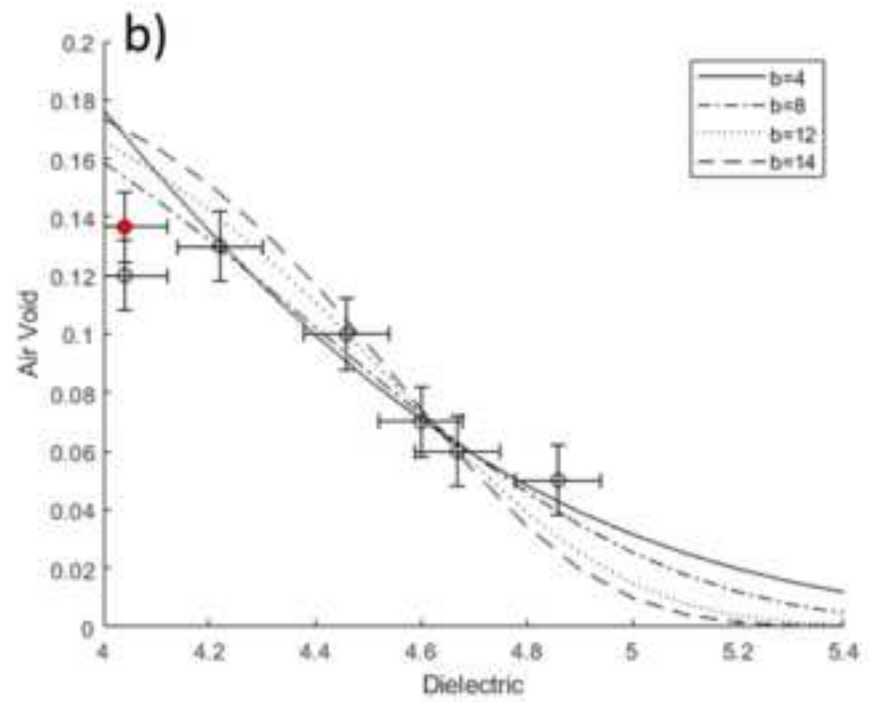
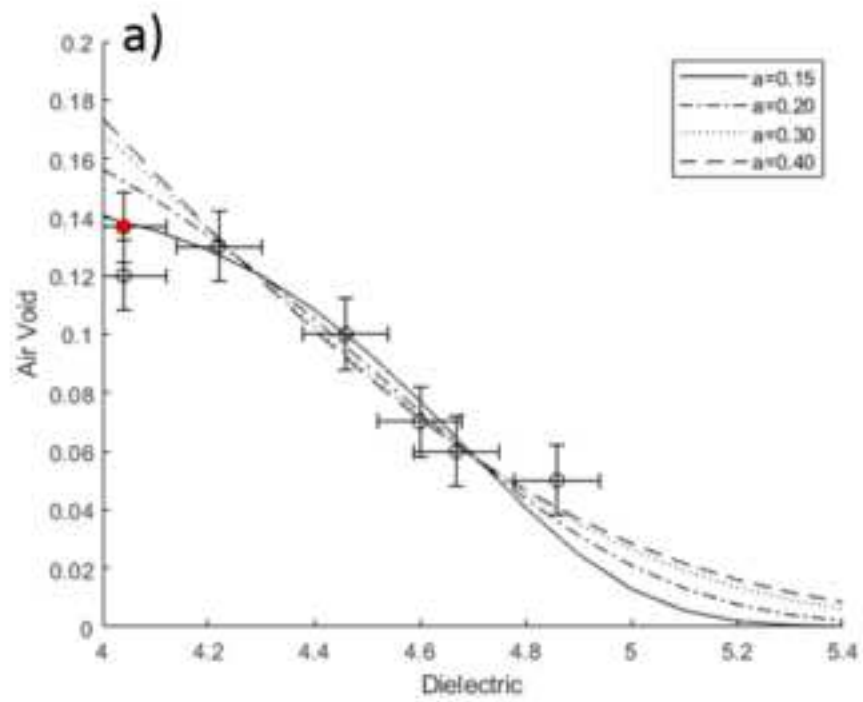












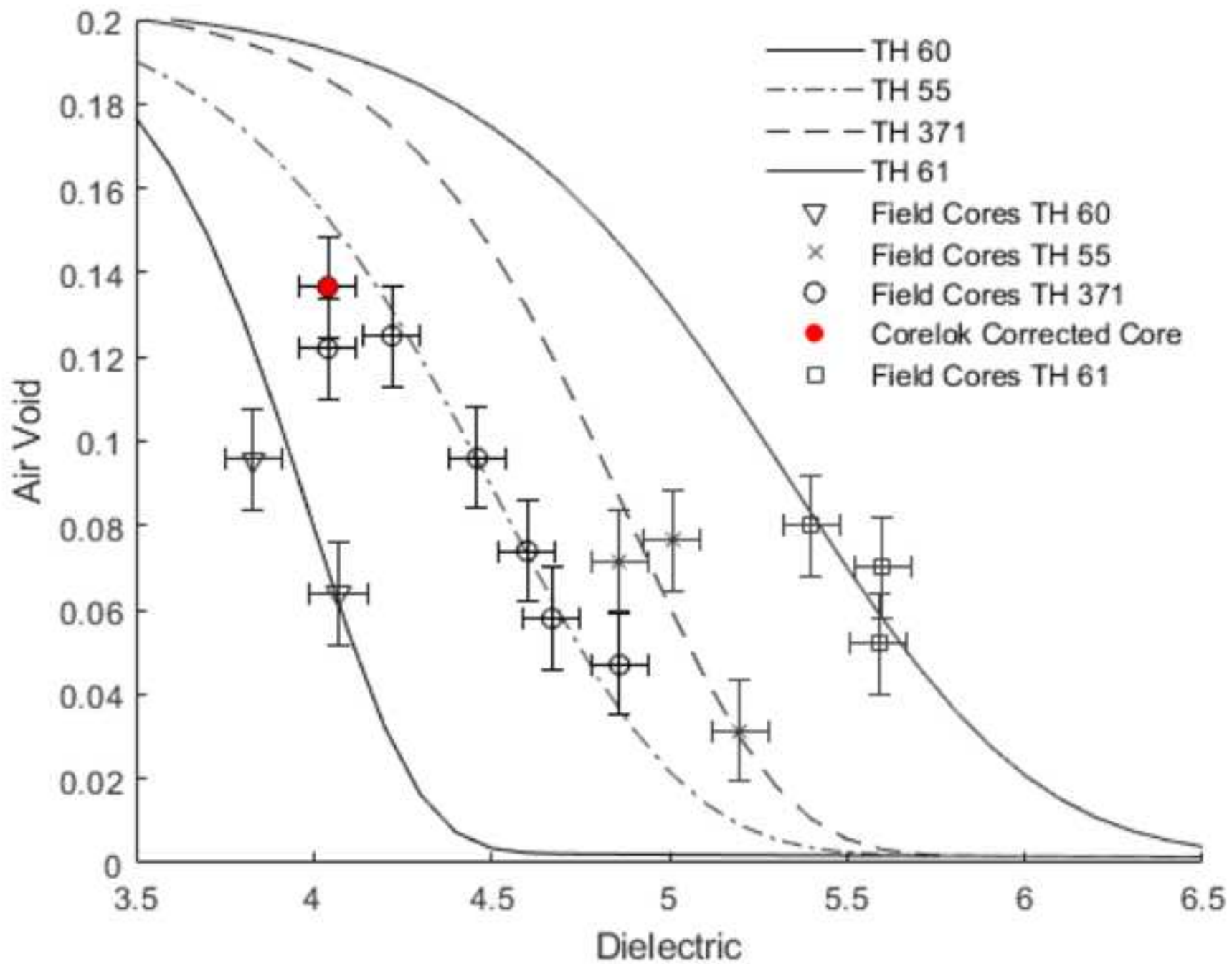


FIGURE CAPTIONS

Fig. 1. Deficiency of the HD model with fitting field core data. 1a) shows the HD model for TH 15 and 1b) shows the HD model for TH 371.

Fig. 2. Comparison of three empirical fits for experimental data collected on highway 371 in Hackensack, MN from October 1, 2019.

Fig. 3. Comparison of fit stability for a Monte Carlo simulation of 1000 puck dielectric and air void measurements. 3a) shows 1000 simulated fits for the MnDOT model, 3b-c) show the same simulated results for the HD and exponential models, respectively.

Fig. 4. 95th percent confidence interval for the Monte Carlo simulation results.

Fig. 5. Model sensitivity to mix design.

Fig. 6. 95th percent confidence interval for the HD and new models across four different days of paving.

Fig. 7. Plots depicting various fits of gyratory puck data at fixed values of four of the fit parameters. 7a-d) show the sensitivity of the MnDOT model to the parameters a , b , c , and g parameters.

Fig. 8. Proposed model fits for TH 371, TH 55, TH 60 and TH 61.